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Abstract: The influence of climate change on the availability and quality of both surface- and ground-water resources is well recognized nowadays. In particular, the mitigation of saline water intrusion mechanisms (SWI) in coastal aquifers is a recurrent environmental issue. In the case of the Nile delta, the presence of sea level rise (SLR) and the perspective of other human-induced stressors, such as the next operation of the GERD (Grand Ethiopian Renaissance Dam), are threats to be taken into account for guaranteeing resilient agricultural practices within the future possible scenarios. Subsurface draining systems can contribute to mitigate the vulnerability to climate change and to the increased anthropic pressure insofar they are able to receive the incremented flow rate due to the foreseen scenarios of SLR, recharge and subsidence. Subsurface drainage offers a practical solution to the problem of upward artesian water movement and the simultaneous downward flow of excess irrigation water, to mitigate the salinization in the root zone. This paper introduces a rational design of subsurface drainage systems in coastal aquifers, taking into account the increment of flow in the draining pipes due to future possible conditions of SLR, recharge and subsidence within time horizons that are compatible with the expected lifespan of a buried drainage system. The approach proposed in this paper is characterised by the assessment of the incremental flow through the drains as a function of various possible scenarios at different time horizons. Our calculations show that the impact on the discharge into the subsurface drainage system under the new foreseen conditions is anything but negligible. Thus, future climate-related scenarios deeply impact the design of such hydraulic structures, and must be taken into account in the frame of the next water management strategies for safeguarding agricultural activities in the Nile delta and in similar coastal contexts.

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Effects of climate change on the design of subsurface drainage systems in coastal aquifers in arid/semi-arid regions: Case study of the Nile delta

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Keywords: sea level rise, coastal aquifer, climate change, subsurface draining systems, seawater intrusion

ABSTRACT:

The influence of climate change on the availability and quality of both surface- and ground-water resources is well recognized nowadays. In particular, the mitigation of saline water intrusion mechanisms in coastal aquifers is a recurrent environmental issue. In the case of the Nile delta, the presence of sea level rise and the perspective of other human-induced stressors, such as the next operation of the Grand Ethiopian Renaissance Dam, are threats to be taken into account for guaranteeing resilient agricultural practices within the future possible scenarios. Subsurface drainage offers a practical solution to the problem of upward artesian water movement and the simultaneous downward flow of excess irrigation water, to mitigate the salinization in the root zone. Subsurface draining systems can contribute to mitigate the vulnerability to climate change and to the increased anthropic pressure insofar they are able to receive the incremented flow rate due to the foreseen scenarios of sea level rise, recharge and subsidence. This paper introduces a rational design of subsurface drainage systems in coastal aquifers, taking into account the increment of flow in the draining pipes due to future possible conditions of sea level rise, artificial recharge and subsidence within time horizons that are compatible with the expected lifespan of a buried drainage system. The approach proposed in this paper is characterised by the assessment of the incremental flow through the drains as a function of various possible scenarios at different time horizons. Our calculations show that the impact on the discharge into the existing subsurface drainage system under the new foreseen conditions is anything but negligible. Thus, future climate-related scenarios deeply impact the design of such hydraulic structures, and must be taken into account in the frame of the next water management strategies for safeguarding agricultural activities in the Nile delta and in similar coastal contexts.

1 Introduction

Climate change is a global phenomenon; however it's very variable on a geographical basis. It is defined as an imbalance in the usual climatic conditions such as heat, wind and rainfall patterns that characterize each region on earth. A number of phenomena is addressed to produce the sea level rise (SLR), and its acceleration that is currently observed, such as the thermal expansion of the oceans and seas, the melting of glaciers and ice caps including Greenland, and Antarctic ice sheets melting. According to the Intergovernmental Panel on Climate Change (IPCC, 2013), in year 2100 about 95% of the coastal areas in the world will be considerably affected by SLR, hence increasing the risk of inundation in internal land and salt water intrusion (SWI) in coastal aquifers (Agren and Svensson, 2017). Shaltout et al. (2015) indicated SLR effects in the Nile delta in terms of ongoing submergence. According to Sestini (1989) and IPCC (2008), Egypt is considered among the most vulnerable countries to the threat taken by SLR.

Recent measurements by both ground based and satellite observations also indicate an acceleration in the rates of SLR (Legeais et al., 2018). El Raey (2010) studied both environmental and socio-economical risks in the coastal zone of the Nile delta connected with climate change, also placing attention to SLR and subsidence. The IPCC in its 5th report (IPCC, 2013) predicts global SLR figures from 18 to 59 cm in the next 100 years. For what regards Egypt, such conditions would lead to the submergence of the low lying coastal zones and some parts of the Nile Delta adjacent to the northern coast. In addition to surface water issues, a particularly challenging aspect is the mitigation of saline water intrusion mechanisms (SWI) in groundwater, in the presence of sea level rise and with the perspective of other human-induced stressors, such as the next operation of the Grand Ethiopian Renaissance Dam (GERD), the overpopulation in the Nile delta and Nile valley, and the various forms of desertification processes currently observed (Aboel Ghar et al, 2004). The risk of increased salinization deeply impacts agricultural practices and must also be taken into account for the management of reclaimed lands.

The Nile delta aquifer is a Quaternary aquifer mainly composed of a Holocene clay cap layer and a series of Pleistocene layers (deepest confined aquifer). The Holocene clay cap acts as an aquitard in its southern part, with an average thickness between 5 and 25 m, while in the northern parts it acts as an aquiclude and its thickness is larger than 50 m (Said, 1962, 1981; Serag EI-Din, 1989). The Pleistocene layers consist of coarse-grained Quartzitic sands and gravels alternated by lenses of clay. Its thickness gradually increases

toward the sea, being about 200 m in the South and sometimes exceeding 950 m in the North (El-Fayoumi, 1987; Said, 1993).

Both the risk of submergence and SWI are exacerbated by subsidence (Wöppelmann et al, 2013). Several mechanisms could be brought as cause of subsidence: the sediment loading and isostatic displacement, or the failure, faulting, and flow of under-consolidated sediments, the anthropogenic or tectonics factors and the sediment compaction. Analysing the scales, the patterns and the velocities observed across the northern Nile Delta, the sediment compaction is the only subsidence mechanism consistent with them. Rapid compaction rates have been recorded in the meter increment below the top 1.0 to 2.0 meters of sediment on the Nile Delta. These rates appear to decrease to around 5.0 to 6.0 meters in an irregular fashion and then continue to decrease more regularly to the base of the Holocene section (Stanley and Corwin, 2012). Much of the subsidence measured by previous studies occurs in conjunction with known Holocene sediment deposits. Therefore, subsidence patterns do appear to correlate with thick Holocene sediment accumulations, but also appear to be heavily influenced by young sediment deposits of less than 3500 years old due to rapid compaction within the first few meters (Fugate, 2014).

Subsidence exhibits variable rates across the Nile delta (Becker and Sultan, 2009; Fugate, 2014), often higher than SLR. In particular, it is expected that vertical land motion has a relevant impact on the assessment of future scenarios and the relating time horizon, regarding the risks of both submergence and salt water intrusion. In particular, the combination of SLR and subsidence has an impact on the design of drainage systems, especially in view of an acceleration of both phenomena.

Laeven (1991) showed that the saline water of the Mediterranean Sea intrudes into the Nile aquifer at depths in the range from 175 to 225 m, in the deepest confined aquifer. Moreover, Sakr et al. (2004) analyzed the historical records from 1960 to 2000 and demonstrated the sensitivity of groundwater salinity with respect to the Nile flow and abstraction rates. Authors concluded that the reduction in the Nile flow and the extensive abstraction from the aquifer lead to an increase in groundwater salinity. The effect of Nile flow reduction, which is expected when the GERD will enter into operation (Abd-Elhamid et al., 2018), combined with the increased pumping of groundwater and the current trend of sea level rise

88 is expected to affect seawater intrusion mechanisms in the Nile Delta in a dramatic way, requiring the
89 urgent identification and design of possible counter-measures.

90 Several studies were carried out on the Nile delta confined aquifer to investigate the possible impact of
91 SLR, including Sherif and Al-Rashed (2001), Sherif et al. (2012), Sefelnasr and Sherif (2014), Abd-Elaty
92 et al. (2014) and Abd-Elhamid et al. (2016). These studies indicated that SLR has a negative impact on
93 saltwater intrusion, predicting a sure threat to a large quantity of freshwater. An alternative strategy to
94 mitigate the SLR effects is the artificial recharge (AR). In the coastal aquifer it reduces the seawater
95 intrusion growing the fresh water pressure in the aquifer. AR requires an increase of alternative water
96 supplies, such as desalination plants and water reuse from one side and aquifer recharge systems on the
97 other side (Abd-Elhamid and Abd-Elaty, 2017).

98 Drainage is an essential practice in agricultural irrigated lands for preventing water-logging and
99 salinization (Abd-Elaty et al., 2010). When the conditions in an area are such that an upward seepage
100 from an underlying aquifer occurs, the risks of water logging and salinity are more serious. Such
101 conditions may prevail in some areas of the Nile delta. Especially the northern coast is expected to be
102 negatively impacted by both SLR and subsidence in the future. Subsurface drainage offers a possible
103 practical solution for avoiding the salinization of the ground in the root zone, in the presence of upward
104 artesian water movement with a simultaneous downward flow of excess irrigation water.

105 The design objective of the drains is to keep the water table within specified limits, determining a flow of
106 water through the soil to the drains. Bazaraa et al. (1986) studied the artesian and anisotropic effects on
107 drain spacing. Subsurface drainage systems installed in a soil overlying an artesian aquifer should be
108 designed to handle both the upward artesian water flow and the downward seepage flow due to irrigation
109 and rainfall. Proper drain spacing and sizing depends on several parameters, and it is known that drains
110 subject to artesian conditions require a narrower spacing with respect to the simplest condition
111 characterized by downward seepage only.

112 In 2011, Kalantari studied the impact of climate change on drainage systems. The study concluded that
113 hydrological models are fundamental tools to assess the discharge dynamics, and the expected changes
114 due to climate factors. Deelstra (2015) indicated that drainage construction is often carried out based on

115 existing practice and experience. Today, this might not be suitable under conditions of climate change,
116 anthropic pressure and increased extreme events. Therefore, factors of influence on the design of drainage
117 systems should be known and taken into account. A deep understanding of the interaction between
118 climate, geological setting, land use, watershed hydrology and groundwater flow is a necessary
119 prerequisite for the design of a draining system characterized by a long-enough service time horizon.

120 The design improvement of drainage systems according to new perspective scenarios characterized also
121 the recent literature related to the North American region. In 2017, Pease et al. used the DRAINMOD
122 hydrologic model to simulate the expected climate change effects on subsurface drainage and the
123 performance of controlled drainage in the western Lake Erie Basin (US), where the climate change is
124 expected to increase both annual rainfall and temperatures. The study indicated that the development of
125 strategies to mitigate the impact of these changes is important for ensuring agricultural resiliency to the
126 future climate. British Columbia (Canada) Agriculture & Food Climate Action Initiative (2013) reported
127 that, in the countries where subsurface drainage is necessary, a proper design of drainage systems must
128 take into account the mitigation of risks related with the climate change.

129 The adaptation of drainage systems design to climate change varies greatly depending on local conditions,
130 in Egypt most of the previous studies did not take into account the effect of climate change on subsurface
131 drainage design, neither the effect of subsidence was considered. This paper represents a first building
132 block for the proper design of subsurface drainage (SD) systems in the Nile delta and in coastal aquifers
133 in general, in the presence of multiple climate-related forcing parameters. In particular, attention is
134 focused on SLR, subsidence, and on the artificial subsurface recharge that is hypothesized in order to
135 reduce the aquifer salinity.

136 This paper introduces a rational design of subsurface drainage systems in coastal aquifers, by estimating
137 the flow in the draining pipes and its possible increment due to the foreseen scenarios of SLR, recharge
138 and subsidence. A novel approach proposed in this paper consists in the assessment of the incremental
139 flow through the drains as a function of various possible scenarios. The next sections will illustrate the
140 assumptions made and the methods used to build the scenarios, and the evaluation of the expected impact
141 on the design of subsurface drainage systems.

142

2 Materials and Methods

2.1 Eastern Nile Delta Aquifer (Case Study Area)

The Eastern Nile Delta aquifer was selected as a case study area to perform the numerical simulations. The study area is bounded by the Damietta Branch at the West, El Manzala Lake at the North, Ismailia Canal at the South and Suez Canal at the East, and its size is about 9500 km². It is located between latitudes 31° 00` and 32° 30`N, and longitudes 29° 30`and 32° 30`E, as shown in Figure 1 (Nosair, 2011).

The aquifer system of the Nile Delta is one of the largest in the world for its areal extension and layer thickness, with a total capacity of about 500 Bm^3 (Sherif, 1999). Its characteristics have been identified by numerous studies, assuming that it's formed by quaternary deposits and considered a semi-confined aquifer as a whole (Al Agha, 2015). The geology of the aquifer includes Quaternary deposits of Holocene and Pleistocene sediments, plus Tertiary deposits including the Pliocene, Miocene, Oligocene, Eocene, and Paleocene sediments. The Quaternary aquifer thickness ranges from 100 m in the South, near Cairo, to nearly 1,000 m at the coast of the Mediterranean Sea (RIGW, 1980). The hydrological strata are composed of sands and gravels (Pleistocene and Holocene) containing few lenses of clays. These are considered the main water-bearing formations (Sherif et al., 2012). The Quaternary base is a clay aquiclude with a slope of about 4 m/km, which is about 40 times the ground surface slope (Serag EIDin, 1989; Said, 1993), as shown in Figure 2. A number of studies were carried out on the Nile Delta aquifer system under different scenarios of pumping rate (Sherif and Al-Rashed, 2001). In general, the most permeable layer has been found at depths between 55 and 150 meters from the land surface (Sefelnasr and Sherif, 2014).

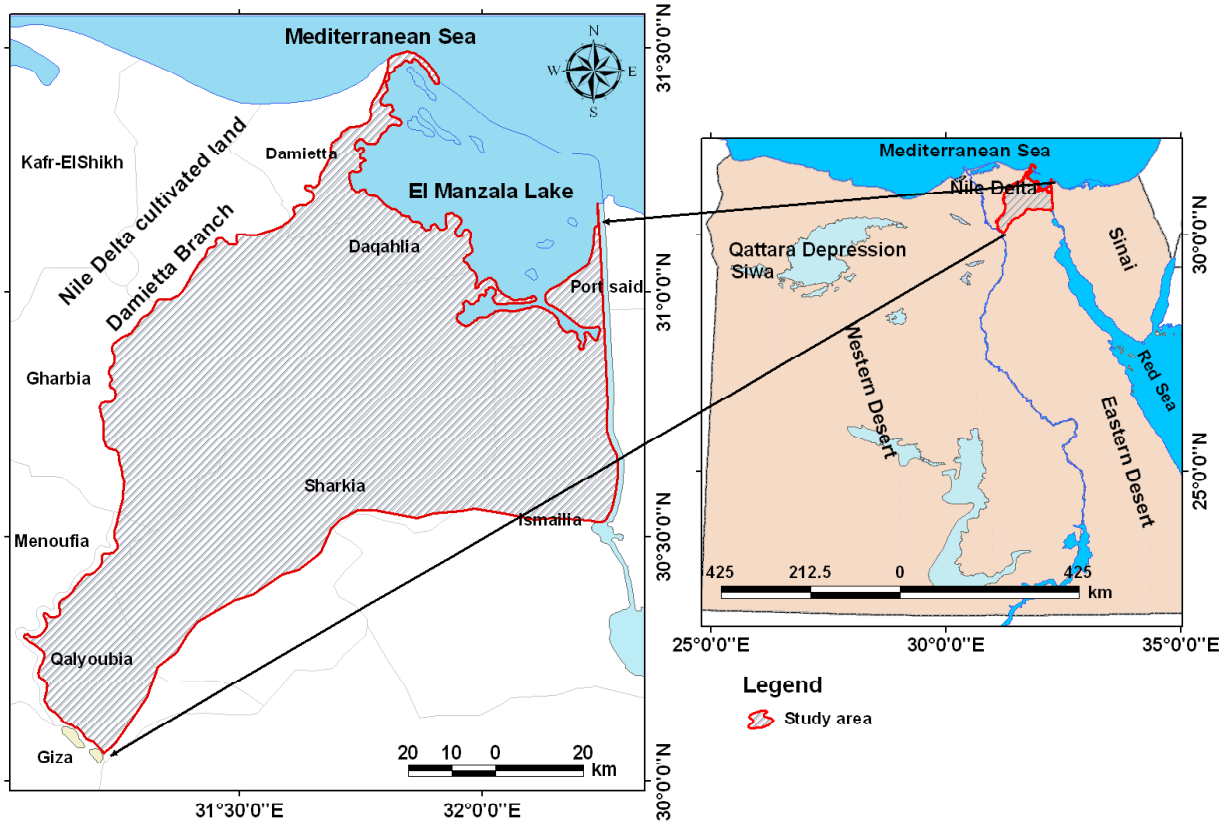


Figure 1: Location map of the East Nile Delta aquifer (from Nosair, 2011)

According to the field data compiled and processed by RIGW (1980), the top soil layer consists of clay, with a thickness that varies generally from 40 m in the North, middle and West Delta region, to about 90 m that are reached at the Damietta branch, with a general increment from West to East. So, according to the thickness of the clay layer, the semi-pervious clay cap admits leakage from the deepest to the shallow aquifer. The northern part of the aquifer is subject to upward flow, due to the difference in head of these water bodies that causes vertical movement of groundwater in the clay cap as shown in Figure 3. The quantity of upward flow in the delta aquifer is almost 50×10^6 m^3/year (Faried, 1979). On the other hand, the main aquifer is recharged by means of the irrigation canals, the seepage from the Damietta branch in Southwest and Ismailia Canal in the Southeast, plus the irrigation water excess. The latest estimates point out an amount of recharge deriving from irrigation practices of about 0.54 mm/day, while the average water loss by evaporation is around 311 mm/year (RIGW/IWACO 1990). According to different estimated depths of the groundwater table (reported by RIGW, 2002 and Morsy, 2009) the depth of the groundwater table in this aquifer ranges between 1- 2 m in the North, 3– 4 m in the center and 5 m in the South.

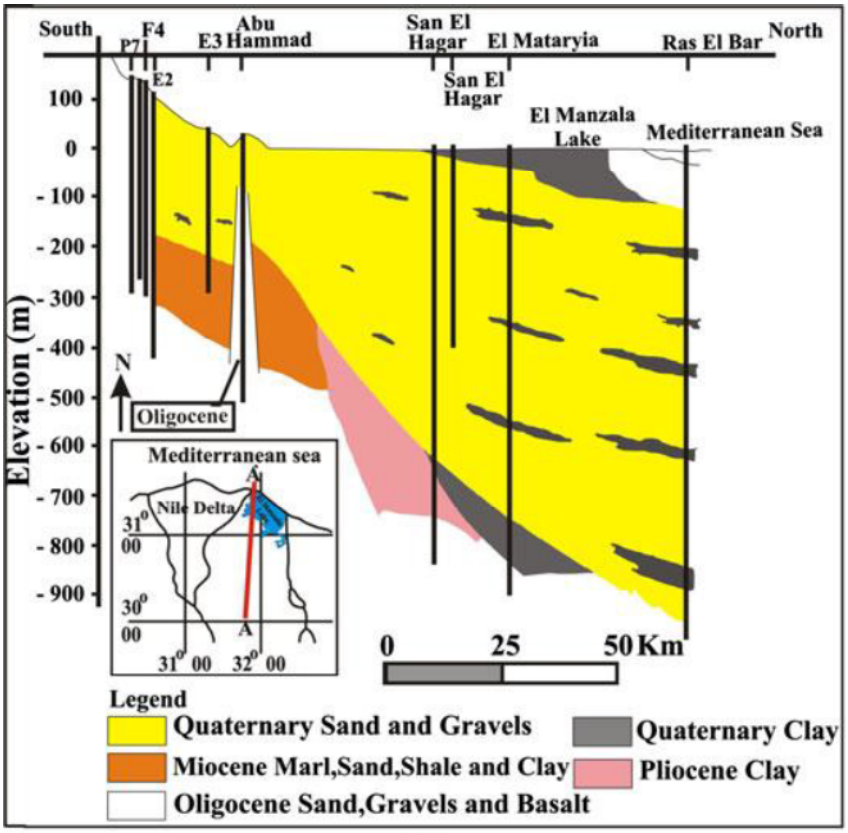


Figure 2: Longitudinal cross-section showing the thickness and litho-facies variation of the Quaternary aquifer in Nile Delta (from El Fayoumy, 1968)

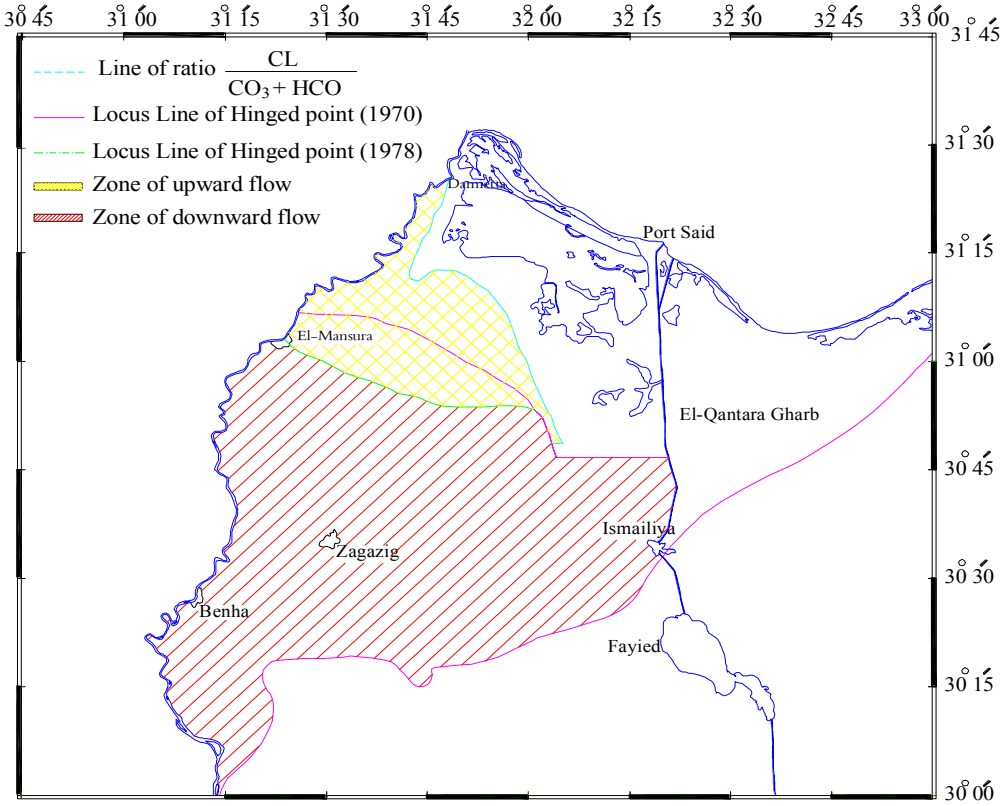


Figure 3: Locus line of the hinge points separating the upward and downward flow zones (After Amer, 1981)

Morsy (2009) estimated the total annual groundwater abstraction in the Nile Delta area to be 4.9 Bm^3 in 2008, for irrigation and drinking purposes. The abstraction rate increases linearly by about 0.1 BM^3 per year, except the period from 2003 till 2010 by rate of 0.2 BM^3 per year (Sallam, 2018). In the study area of this paper (the eastern Delta) the amount of groundwater used in agriculture is about 326 Mm^3/year (Abu-Zeid, 1991) while the total amount reached 1.38 Bm^3 in 2008. The main contributions to groundwater discharge are represented by subsurface drainage and overexploitation of the aquifer. According to Fawzi & Kamel (1994), the groundwater is discharged into the drainage system at a rate of 1.0 mm/day in all the northern (coastal) part of the Nile delta.

In 1979, Atta analyzed the salinity of groundwater in the Nile delta based on the sampling of 50 wells, finding values between 227 ppm and 15264 ppm. The salinity generally increases towards the northeastern zone, while the northern parts and the areas adjacent to the Nile River canals have lower salinity. Farid (1980, 1985) presented maps of salinity distribution, where iso-salinity lines in terms of total dissolved salts (TDS) range from 640 to 45,000 ppm, and vertical cross sections with iso-salinity lines (TDS) ranging from 1,000 ppm to 35,000 ppm. These results agreed with (Atta, 1979) and indicated that the northern zone is highly saline due to saltwater intrusion. Morsy (2009) analyzed and presented the groundwater salinity from 1960 to 2008 based on historical groundwater quality data taken from the literature and from the Research Institute for Groundwater of the Egyptian National Water Research Center (RIGW) database for wells at depths from 30 to 135 m. These analyses confirmed the results shown by Sakr et al. (2004), i.e., the depths at which higher groundwater salinity was found confirm the existence of a clay layer with low permeability over the deepest confined aquifer. Such clay cap is above the depths where Laeven (1991) found the intrusion of saline water. Interestingly, Sherif et al. (2012), Abd-Elaty et al, (2014) and Abd-Elhamid et al, (2016) simulated and described that the seawater in the Nile aquifer migrated to a distance range from 48 to 76.25 km and from 72.50 to 93.75 km from the shoreline for the iso-salinity lines (TDS) at 35000 ppm and 1000 ppm, respectively.

The surface water network (Nile branches, drains and irrigation canals) has a fundamental function to determine the hydrogeological conditions in the northern part of the East Nile Delta area. The subsurface drainage system protects the irrigated soils against salt accumulation and enables recycling of the

irrigation water. It consists of an extensive drainage network of field drains, sub-collectors, collectors and main drains (Figure 4), which either convey the drainage water back to the Nile, or discharge into coastal or inland lakes or directly to the sea (MWRI, 2013).

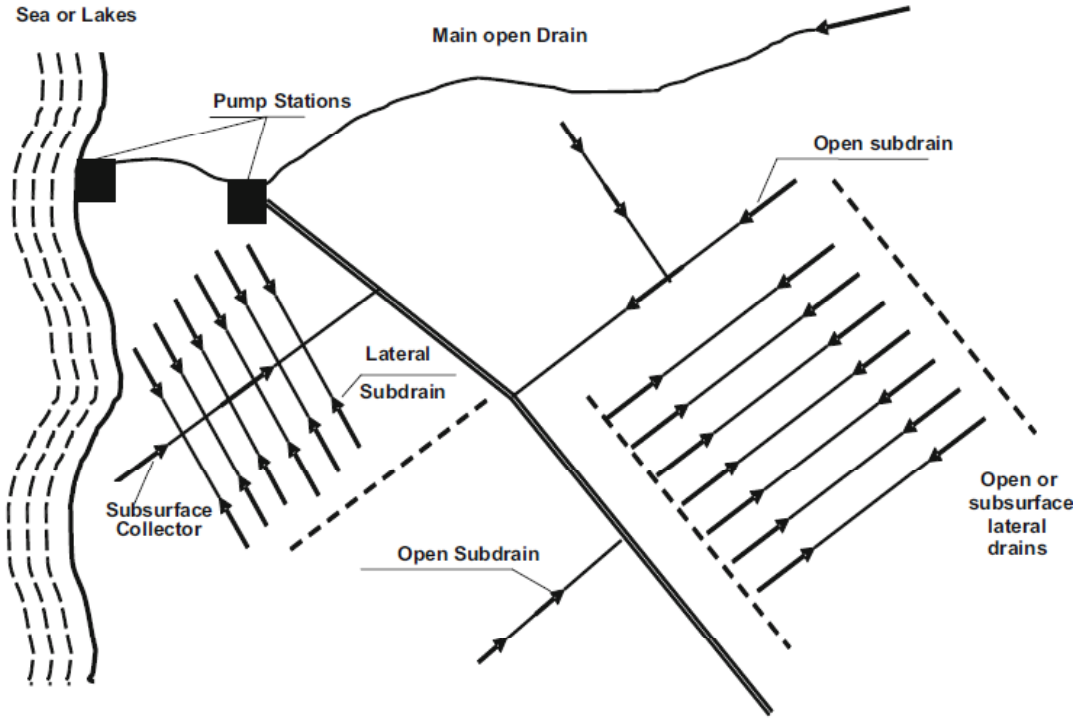


Figure 4: Schematic diagram of Nile delta surface and subsurface drainage networks

2.2 Description of the Numerical Model

The numerical model used to simulate the seawater intrusion in the Nile delta aquifer is based on the assumption of phreatic coastal aquifer subject to a top-down infiltration due to recharge (rainfall plus irrigation) and a bottom-up flow through a confined aquifer.

Seawater intrusion phenomenon is a miscible variable density process governed by the following coupled system of flow and transport equations:

$$\phi\frac{\partial\rho}{\partial t}-\nabla\left(\frac{\rho\boldsymbol{K}}{\mu}(\nabla p+\rho g\nabla z)\right)=0$$
$$\phi\frac{\partial(\rho C)}{\partial t}+\nabla(\rho\boldsymbol{q}C)-\phi\nabla(\rho\boldsymbol{D}\nabla C)=0$$

(1)

where $\boldsymbol{K}$ is the hydraulic conductivity tensor; p is pressure; ϕ is porosity; ρ and μ respectively are fluid density and viscosity; g is the gravitational constant; C is the solute (salt) concentration; $\boldsymbol{D} = (\phi d+\alpha_T|\boldsymbol{v}|)\boldsymbol{I} + (\alpha_L - \alpha_T)\,\boldsymbol{v}\,\boldsymbol{v}_T/|\boldsymbol{v}|$ is the hydrodynamic dispersion tensor and d diffusion coefficient; α_T and α_L respectively are transverse and longitudinal dispersivity; $\boldsymbol{v} = \boldsymbol{q}/\phi$ is the fluid velocity, and the superscript T denotes transpose. System (1) is closed by specifying a constitutive relationship, $\rho = \rho_f + \beta C$, where $\beta = (\rho_s - \rho_f)/C_s$, ρ_s and ρ_f being salt and freshwater density respectively, and C_s the saltwater concentration.

2.2.1 Regional scale numerical model: Seawater intrusion in the Nile delta aquifer

The latest version of SEAWAT (Langevin et al., 2008), a coupled version of MODFLOW and MT3DMS to integrate the density-dependent flow and the solute transport equation, was used to simulate seawater intrusion in the Nile delta aquifer at a regional scale.

Subsurface flow and solute transport in the main aquifer was modelled by using 160 rows and 124 columns of active cells, with a cell dimension of 1.0 × 1.0 km^2. The Nile Delta aquifer was divided into eleven layers. The first layer represents the clay cap with depth varied between 20 m in the North to 50 m in the North. The other layers, which represent the Quaternary aquifer, were divided into slices of equal thickness, until an average depth of 200 m near Cairo to a depth of 1000 m at the coast line.

A constant head boundary condition was set equal to zero at the northern boundary along the shore line, also at the western boundary a fixed head was imposed, between 16.15 m at the North and 0.25 m at North. On the other hand, the domain is bounded to the South by the Ismailia canal, with a variable water level between 16.15 m at its westernmost point and 7.00 m at its easternmost point, so a Dirichlet boundary condition was assigned. The East boundary was considered impermeable and a no-flow (Neumann) boundary condition was set. During the simulation, the hydraulic head boundaries along the Nile branches were assumed as constant. The hydrodynamic parameters fed to the model are shown in Table 1.

Table 1: Summary of hydraulic parameters used as an input to the model

Hydraulic Parameters	**Value**
Vertical Quaternary hydraulic conductivity K_v(m/d)	0.50 - 10
Horizontal Quaternary hydraulic conductivity K_h(m/d)	5 - 100
Vertical clay cap hydraulic conductivity K_v(m/d)	0.01 - 0.025
Horizontal clay cap hydraulic conductivity K_h(m/d)	0.10 - 0.25
Porosity	0.25 – 0.40
Longitudinal dispersivity (α_L) (m)	250
Transversal dispersivity (α_T) (m)	25
Diffusion coefficient (d) (m^2/day)	10^{-4}
Hydraulic Forcing	**Value**
Recharge (mm/day)	0.25 - 0.80
Total abstraction (m^3/ year)	2.78×10^9

In order to obtain realistic results, the numerical model of the Nile Delta aquifer was calibrated by using available historical records, consisting in hydraulic head measurements in a number of piezometers distributed in the studied area, during a field campaign in 2008 (RIGW). The calibration of the numerical model was based on the comparison between modeled and measured hydraulic heads, by modifying the values of hydraulic conductivity, of porosity and of aquifer recharge rate, in order to optimize the match between the modeled and observed heads.

After calibration of the model, the maximum difference between measured and modeled heads (ranging from 16.00 to 0.0 respectively), is about 10 %, corresponding to about 1.60 m in the southernmost point. Comparing the calculated heads with the observed measurements in the Nile delta aquifer, the root mean square (RMS) of the residuals was equal to 0.329 m with a residual range between -0.215 and 0.488 m and a normalised RMS of 2.744 % (i.e. normalized with respect to the maximum difference in the observed head values). The results of the numerical model, in terms of hydraulic head (Figure 5), are in good agreement with the observed heads described in RIGW (2008).

A transport model has also been used to determine sea water intrusion (SWI) in the Eastern Nile delta aquifer. Firstly, it was calibrated by using field data of saltwater intrusion published by Sherif et al. (2012). The hydrodispersive parameters (i.e. α_L and α_T) contained in equations (1) have been calibrated by

comparing saltwater concentrations measured by Sherif et al. (2012) with modeled concentrations. Calibration results are presented in Figure 6, which shows the distribution of total dissolved salts (TDS) in the middle of the aquifer (layer #6), assigning an average thickness of 450 m at the North and 100 m at the South. These values are in good agreement with the field data of SWI in the Nile delta aquifer by Sherif et al (2012). The isochlorine at 35000 ppm reaches a distance of 75.85 km from the shoreline, while the isochlorine at 1000 ppm reaches a distance of 90.40 km.

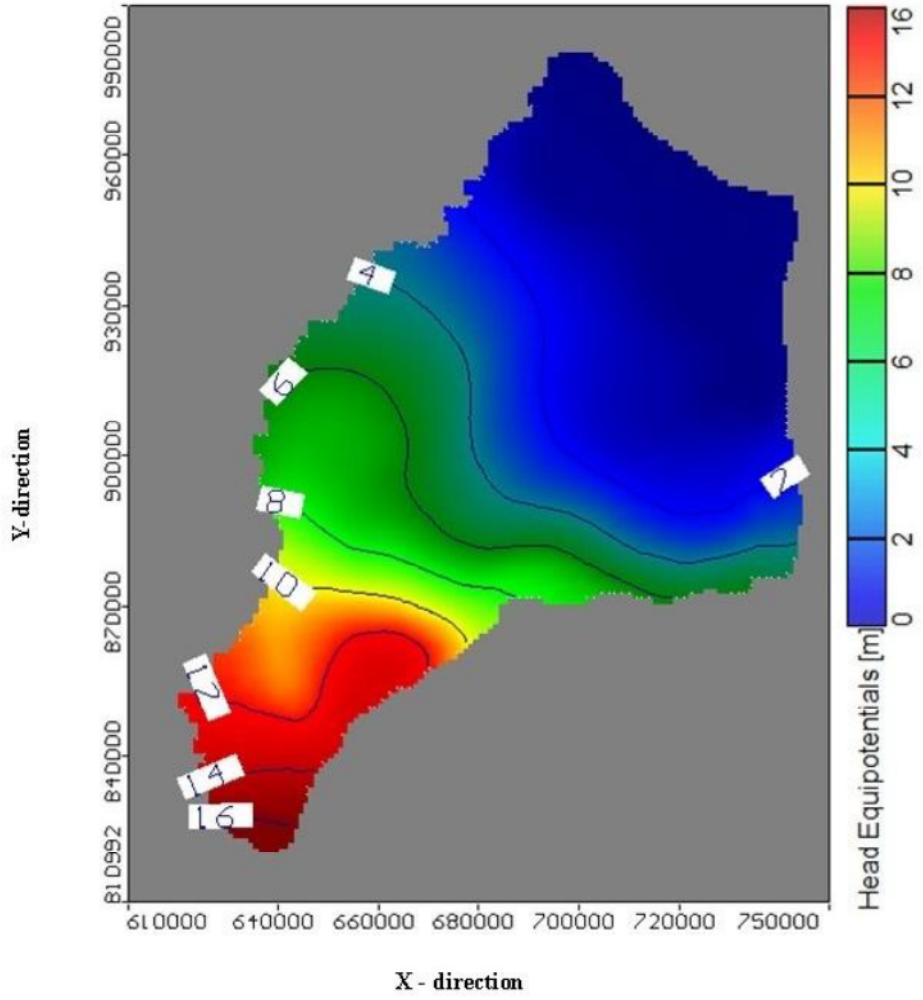
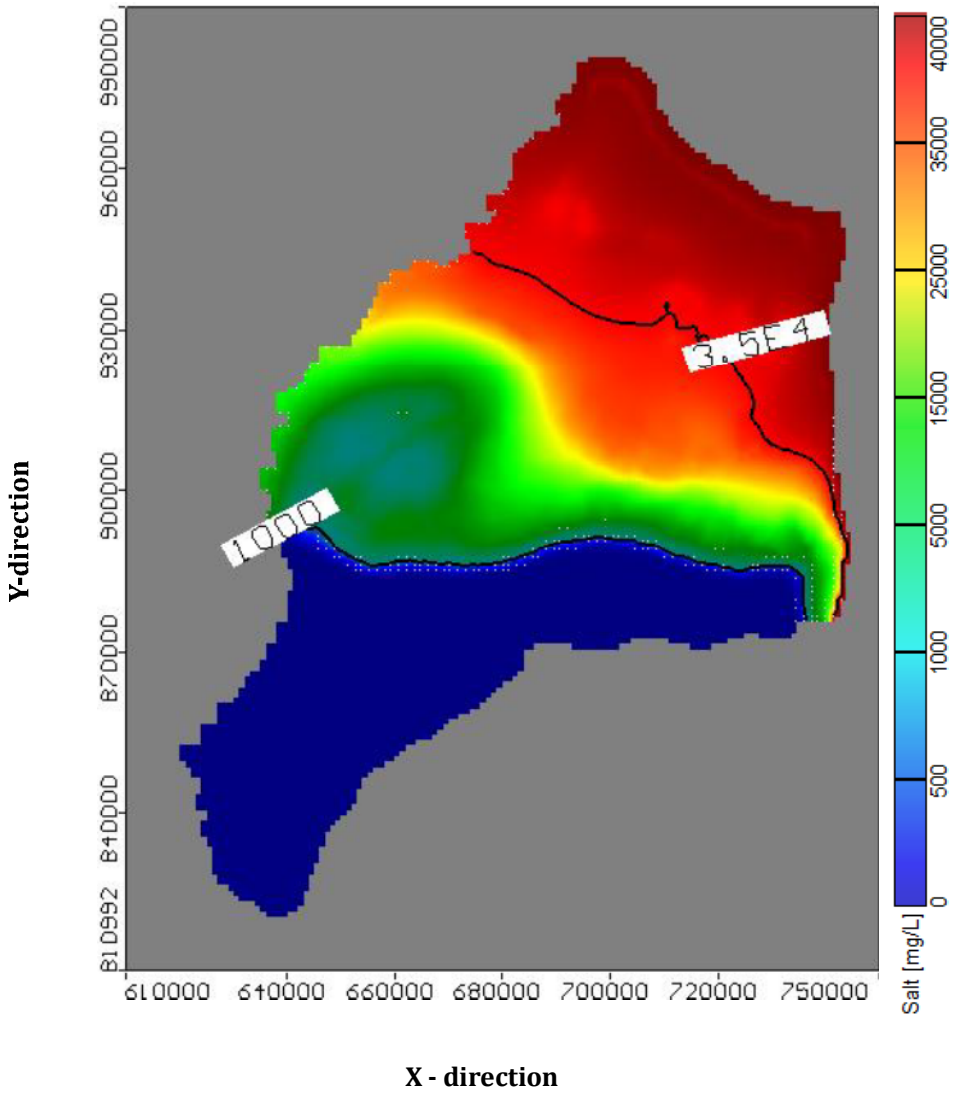


Figure 5: Map of calculated groundwater head (simulation model) in the studied aquifer



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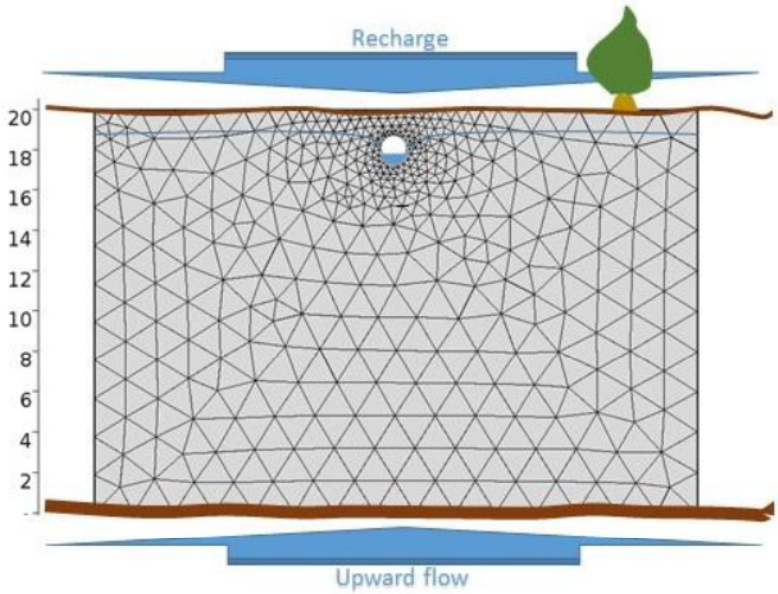
278 Figure 6: Map of TDS (output by SEAWAT) in the studied aquifer

279 *2.2.2 Local scale numerical model: subsurface drain discharge*

280 In some areas of the Nile delta, especially in the North, the risk of water logging and salinisation due to
281 the upward seepage from the underlying aquifer is very serious. Subsurface drainage systems, installed in
282 the soil overlying artesian aquifers, offer a practical solution to the problem of upward artesian (brackish)
283 water movement. The drain size and spacing need to be properly calculated (Abdel-Dayem, 1984) to
284 make the drainage system able to discharge the excess of irrigation water as well as the upward
285 groundwater flow. In practice, subsurface drains must be designed to withstand the necessary water flow,
286 which permits to tie the water table to the required depth, for any foreseen working condition that may
287 happen within the lifetime of the hydraulic structure. Thus, in the particular case of the Nile delta and of
288 coastal aquifers in general, the SD design must take into account the effects of SLR, subsidence, and all

289 the human actions on the groundwater flow (e.g. over-pumping, surface flow reduction, artificial
290 recharge, etc.). This is a mandatory effort to be done, in order to guarantee resilience of agricultural
291 practices towards possible climate change scenarios. It must be noted that this effort has been
292 substantially underestimated until today.
293 The hypothesized geometry for the draining system considered in this study consists in a series of
294 identical parallel drains, regularly distributed with an equal spacing distance, orthogonal to a main
295 collector.
296 A Finite Element Model was implemented in the COMSOL Environment (COMSOL, 2008) in order to
297 simulate the response of the subsurface drainage system to the actions of SLR, artificial recharge and
298 subsidence. The numerical model has a domain size of 20.0 × 30.0 m^2 discretized with 5000 irregular
299 elements (Figure 7). The main hydrological parameters used for the simulation of the drain system
300 geometry are the hydraulic conductivities ($K_h = 1.83\times10^{-6}$ m/s = $10\times K_v$) and the porosity ($\phi = 0.3$). The
301 main forcing acting on the model is the hydraulic head variation ΔG (cm) observed at the drain level due
302 to SLR, recharge increment and subsidence.



303
304 Figure 7: Design criteria of the drainage system at an artesian aquifer
305
306 This model is used to calculate how the flow rate into the drains is modified by different working
307 conditions, linked to the said climate-related scenarios, for the fixed geometry (in particular, given the
308 distance between adjacent drains and their underground depth). By analyzing a set of possible future

309 combinations of SLR, subsidence and recharge within the next tens of years, useful estimations for an
310 optimal performance of the drainage system can be done.

311 ## 2.3 SLR datasets

312 The estimation of SLR is carried out by analyzing the time series of tide gauge measurements (sometimes
313 longer than 100 years), which are today completed by satellite radar altimetry measurements (about 25
314 years of global data at the time of writing this paper). The combination of advanced radar altimetry
315 techniques, GPS and Synthetic Aperture Radar (SAR) interferometry with the persistent scatterer
316 technique (Crosetto et al, 2016), permits to cross-validate such challenging measurements and separate
317 the contributions of SLR and vertical land motion (VLM) to tide gauge measurements.

318 For what regards the mere SLR estimation on the coasts of the Nile delta, Essink and Kleef (1993)
319 indicated 60 cm as a reasonable estimate within a time horizon of 100 years. The current estimation of the
320 average global SLR derived from satellite altimetry is about 3.3 mm/year with an acceleration of 0.1
321 mm/y^2 (Legeais et al., 2018), and it's very variable on a geographical basis, as stated by the ESA Sea-
322 Level CCI (Climate Change Initiative) project (ESA SL-CCI, 2018). Actually, one of the main issues of
323 satellite radar altimetry lies in the extraction of accurate sea level estimations when approaching the
324 coasts (Vignudelli et al., 2011). In the absence of a global coastal sea level product, near-shore
325 measurements at the northern coasts of the Nile delta would require a specific study, which falls outside
326 the scope of this paper. Instead, gridded data of global mean sea level (for open ocean studies) have
327 already been produced by the altimetry community and can be used for rough estimates, provided that
328 datapoints closest to the coasts are usually few km off the shoreline (the spatial resolution is 1/4 of
329 degree). Based on the regional mean sea level trend map published by the CCI project, the SLR in front of
330 the Nile delta coasts can be roughly estimated in 4 mm/year. A deeper discussion of SLR rate and
331 acceleration, with an adequate approximation for the purpose of this paper, is provided in Section 3.1.

332

3 Results and Discussion

3.1 The effect of SLR and recharge on the coastal aquifer and its quantification

The objective of this section is to estimate the effects of a hypothetical future SLR on the piezometric head and its impact on the SD system by means of the regional numerical model. All the following evaluations are made in terms of foreseen scenarios used to determine the hypothesized inputs to the numerical models. Thus, hypotheses are done for a structure built in year 2000 (taken as a time reference), and scenarios are calculated relating to years 2020, 2040 and 2060, in order to better understand the possible impact of such phenomena within a time horizon compatible with the life expectancy of the buried draining structure..

SLR values for the three time horizons selected were determined according to the most recent available information at the time of writing this paper. Being the fundamental aspects of SLR estimation already introduced in Section 2, here we briefly discuss the reasoning behind the values fed as an input to the modelling exercise.

A constant SLR rate has been assigned to the eastern shoreline of the Nile delta, by averaging selected grid points from the sea-level CCI data base (ESA SL-CCI, 2018), which estimates mean sea level variations by combining multiple satellite altimetry missions. By averaging the closest grid points to the interested shoreline (7 points total) we obtained an SLR rate of 3.20 mm/yr. The cited SL-CCI data are referred to year 2016, thus, cumulated mean sea level variations with respect to year 2000 (for the selected years 2020, 2040 and 2060) were calculated by introducing an additional acceleration to the said SLR rate. The most updated global SLR acceleration rate is calculated in (Nerem et al., 2018), where the authors estimate a "climate-change–driven" acceleration of 0.084 mm/y^2.

Based on these assumptions, SLR values of 5.7 cm, 14.5 cm and 26.7 cm (referred to year 2000) were estimated for the years 2020, 2040 and 2060, respectively.

The results of the regional numerical model show an advancement of the sea water intrusion (SWI) in all three cases. Specifically, the isochlorine at 35000 ppm reaches a distance from the shoreline of 76.25 km, 76.50 km and 77.05 km respectively (it was 75.85 km in year 2000), while the isochlorine 1000 ppm reaches a distance of 90.60 km, 90.75 km and 90.85 km, respectively. These results confirm that SLR

leads to increase SWI in the whole aquifer with a southbound propagation, thus carrying an increased salinity of the water and soil in the root zone, which is negatively impacting the crop productivity starting from the coastal areas. The salt volume in the Eastern Nile Delta aquifer reaches 3.1288×10^{13}, 3.124937×10^{13}, 3.123316×10^{13}, and 3.2116832×10^{13} Kg for a SLR of 0 cm, 5.7 cm, 14.5 cm and 26.7 cm, respectively. The percentages of aquifer salt volume were increased by 1.50%, 2% and 4%, confirming that SLR leads to increase the salt volume in the aquifer.

An artificial recharge scheme is thus hypothesized, in order to counteract the incremented SWI due to SLR. To do that, the approach followed in this research applies a recharge upstream the Nile delta (i.e., in its northern part) at variable percentage rates. This permits to foresee more complete and effective scenarios for the future design of subsurface draining systems and for the assessment of the draining systems that are currently active. A total of 9 runs of the regional model were carried out, in order to simulate all the combinations between SLR and the proposed recharge rates to control the SWI.

Table 2 summarises the model results, taking into account both the expected SLR and 3 possible recharge scenarios, calculated in order to have an inversion of the intrusion process with the lowest value of recharge. The results in terms of intrusion length and aquifer salt volume (where C_0 is the initial salt concentration and C is the salt concentration at the given combination of SLR and recharge) are also shown in Table 3.

The table presents the percentage (%) of recharge required to keep the intrusion at the base case for three cases of SLR so the percentage of required recharge are 1, 1.50 and 3% at SLR of 5.70, 14.50 and 26.70 cm while the intrusion reached 90.34, 90.36 and 90.34 for the isochlorine 1000 ppm while it reached 75.79, 75.82 and 75.75 ppm for the isochlorine 35000 ppm. Also the aquifer salt variation $(C-C_0)/C_0$ were calculated to check the aquifer salt situation which the positive sign indicated that the aquifer salt is more than the base case and this is a negative impact to salt remove while the negative sign represents the recharge have a positive effect on saltwater intrusion.

Table 2: Regional model results as a function of SLR and various recharge levels

Time (year)	SLR (cm)	Recharge (%)	Intrusion length (km)		Aquifer salt variation $(C-C_0)/C_0$		Aquifer salt removal effect
			1000	35000	1000	35000	
2000	0	0	90.40	75.85	0	0	-

		0.50	90.37	75.87	+0.03	+0.02	negative
2020	**5.7**	**1**	**90.34**	**75.79**	**-0.07**	**-0.08**	positive
		1.50	90.30	75.72	-0.11	-0.17	positive
		1	90.39	75.90	-0.01	+0.06	negative
2040	**14.5**	**1.50**	**90.36**	**75.82**	**-0.04**	**-0.03**	positive
		2	90.33	75.75	-0.08	-0.13	positive
		2	90.4	75.90	0.00	+0.06	negative
2060	**26.7**	**3**	**90.34**	**75.75**	**-0.07**	**-0.13**	positive
		4	90.27	75.61	-0.14	-0.32	positive

It is clear that the three stages of SLR lead to increase groundwater level and SWI in the aquifer, as expected. This rising would damage a relevant quantity of freshwater in the aquifer, increasing the salinity of the soil and groundwater in the root zone.

According to the model results, the control of SWI and soil salinity in the root zone under the hypothesized SLR conditions (5.7 cm, 14.5 cm and 26.7 cm) would be performed by artificially increasing the recharge by 1%, 1.5% and 3% respectively. This will lead to decrease SWI but increase groundwater level. Figure 8 shows the relationship between piezometric head difference and the distance from the shoreline under different SLR values and different scenarios of recharge. Figure 9 shows that the maximum difference in head occurs at the South, due to the maximum values of recharge applied there. The piezometric heads in the aquifer in the case study area under the different scenarios of recharge are shown in Table 3, which will be fully discussed later in this paper.

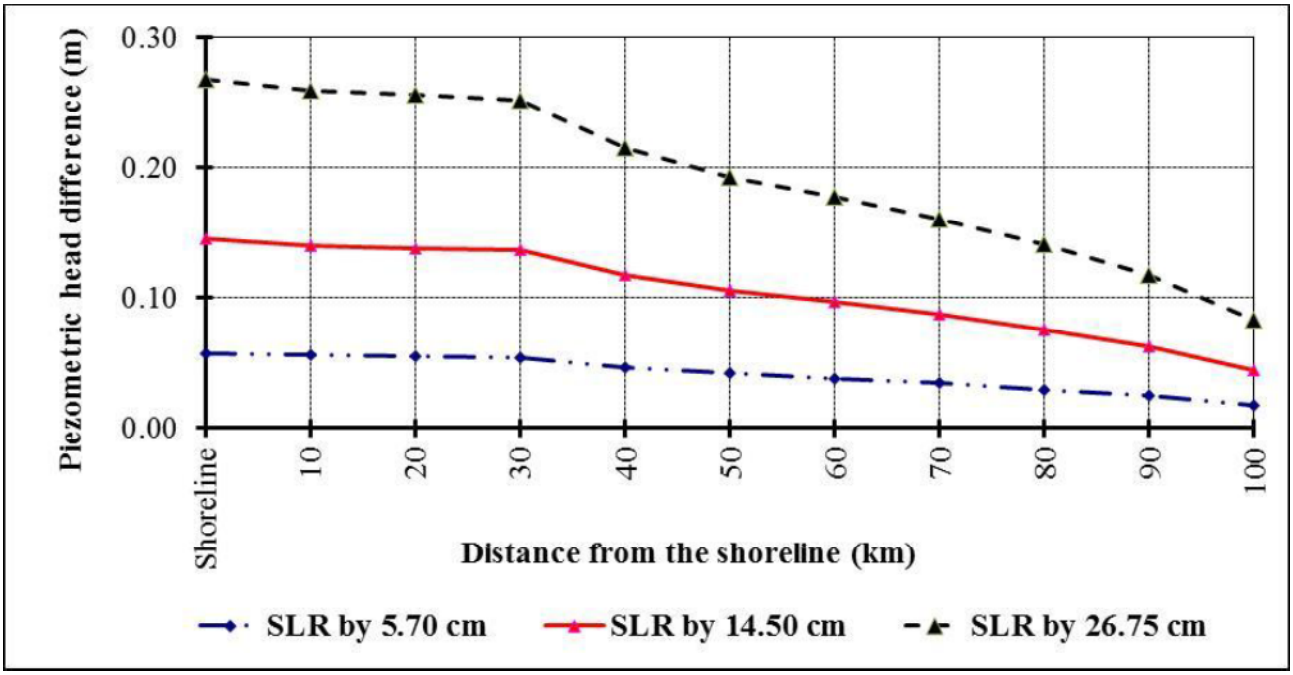


Figure 8: Relation between piezometric head difference and distance from the shoreline for the proposed scenarios of SLR

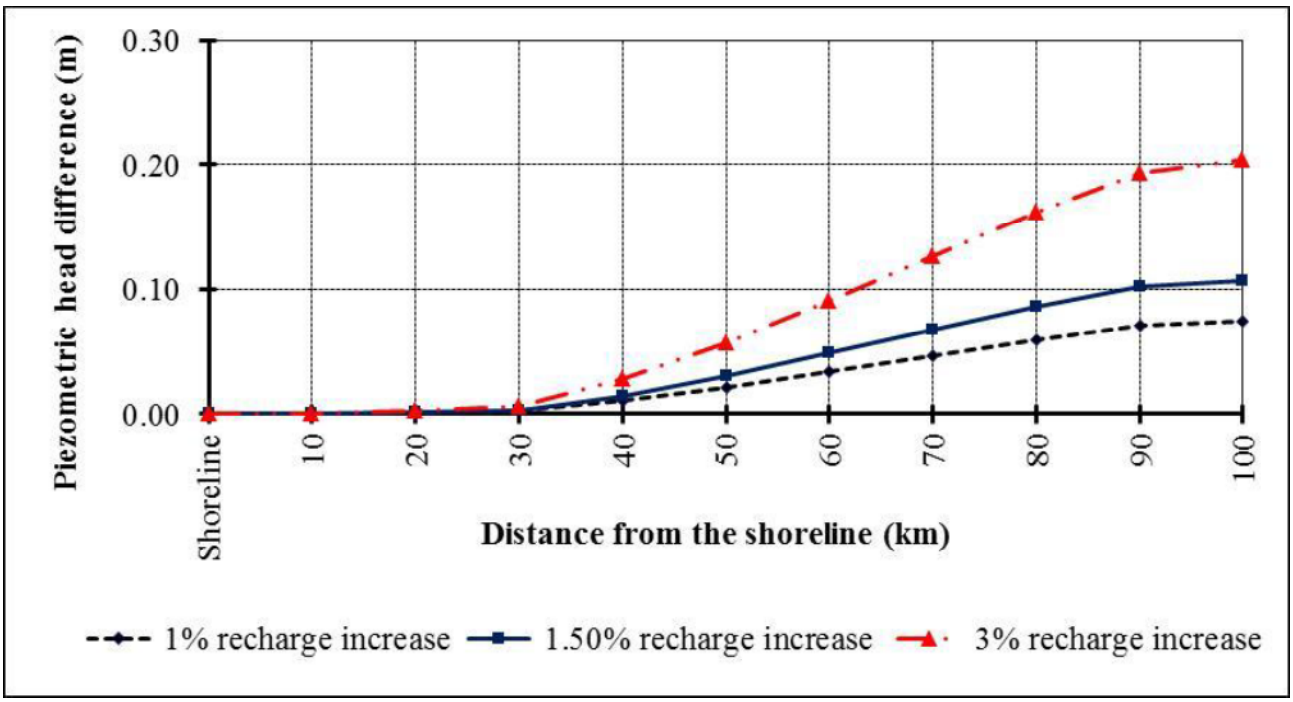


Figure 9: Relation between piezometric head difference (G) and distance from the shoreline according to the three proposed scenarios of SLR and recharge

3.2 The effect of subsidence on the coastal aquifer and its quantification

It is clear how a proper design of subsurface draining systems contributes to mitigate the vulnerability to climate change and to the increased anthropic pressure. The main parameters that have been considered until now are the sea level rise and the increased seepage from recharge. In the particular case of the Nile delta, it is important to analyse also the effect of subsidence on the estimation of flow into the drains. The expected mechanism by which subsidence may affect the sizing of the drains consists in the reduction of distance between the semi-confined and the phreatic layers, due to compaction of the volume in between, and, consequently, the hydraulic head gradient between them.

An estimation of the current subsidence rates can be based on recent literature, where the vertical land motion in the Nile delta coasts is measured by SAR interferometric techniques, by using the "persistent scatterer" (PS) approach (Wöppelmann et al, 2013; Becker & Sultan, 2009). Few algorithms have been developed, in order to extract the Line Of Sight (LOS) deformation from SARIn data, and subsequently the vertical component of such deformation. To probe deeper into this technology, the interested reader

may find relevant examples of said techniques in (Ferretti et al. 2001, Berardino et al. 2002, Hooper et al. 2004).

Recent estimates of subsidence rates in the Nile delta are provided by Fugate (2014), very useful for the purpose of this paper even if mostly focused on the northwestern part of the delta. The work by Wöppelmann et al (2013) combined the InSAR acquisition with GPS measurements, with an interesting discussion of their relevant geodetic findings. The authors observed a very low rate of subsidence (about 0.5 mm/yr) in the Alexandria coastal region, with a good agreement between InSAR PS technique and the tide gauge station in Alexandria. According to the authors, higher rates of about 5 mm/yr were observed in the northeastern part of the Nile delta. Becker and Sultan (2009) found subsidence rates up to 8 mm/yr in the northeastern coastal region, with relatively lower rates (4 to 6 mm/yr) around the Manzala lagoon. Fugate (2014) substantially confirmed these velocities, finding subsidence rates around 8 mm/yr with maximums about 10 mm/yr, in an area that covers a substantial part of the study area of this paper (Eastern Nile delta aquifer).

All authors observed high spatial variability of ground motion velocities, characterized by a very irregular spatial distribution, also in the presence of both uplift and subsidence phenomena in nearby zones. Despite that, a general trend to higher subsidence rates is clearly asserted for what regards the northeastern coastal region, with respect to the central and western ones. It is also important to underline here that the next filling of the GERD is expected to negatively affect vertical land motion velocities in the Nile delta, due to the lower water levels that will be experienced in the canals during the filling period and its possible direct geotechnical implication.

For the purpose of this paper, we assigned a constant subsidence rate of 8 mm/yr to the whole study area, as a cautious value to understand and better define the possible scenarios and their impact on the design of the drainage infrastructure. Table 3 in the next section shows the incremental contribution of subsidence to the flow into the SD system, combined with the other influencing factors (SLR and total recharge).

3.3 Quantification of the impact on the subsurface drainage system design

The previous sections demonstrated that the maximum increase of the piezometric head due to SLR occurs in the northern part of the study area, where the aquifer is directly connected with the sea. Also salinity experiences its maximum increase due to SLR on the northern coast, gradually decreasing toward the South. Given this spatial trend, Table 3 shows the values of the predicted piezometric heads (ΔG), calculated at a predetermined distance to the shore line (about 40 km) within the study area.

Given the foreseen increase of the piezometric heads, the impact of the increase of the piezometric heads (ΔG) on the SD performance could be calculated by using local scale numerical model described in section 2.2.2. More specifically, the subsurface drain discharge was calculated towards the predicted piezometric heads according to the different stressors, separately: i) SLR; ii) recharge increment and iii) subsidence. This has been done in order to make clear to the reader the importance of each contribution.

Figure 10 illustrates the hydraulic head distribution around the drains obtained with local numerical model, in the aquitard and in the phreatic aquifer in the given conditions. Table 3 summarises the simulation results for the proposed scenarios. In particular, the table shows the incremental contributions of SLR, recharge and subsidence to the flow into the SD system (named $Q_{SLR,}$ Q_R and Q_{Sub}, respectively). SLR values of 5.7 cm, 14.5 cm and 23.75 cm were respectively assigned to the years 2020, 2040 and 2060, and entered in the numerical model (based on Eq. 1). As a consequence, piezometric heads due to SLR (ΔG_{SLR}) are calculated in 4.65, 11.70 and 21.50 cm, respectively. The estimated increment of discharge Q_{SLR} resulted about 0.81, 2.05 and 3.76 L/day·m for the three consecutive time horizons.

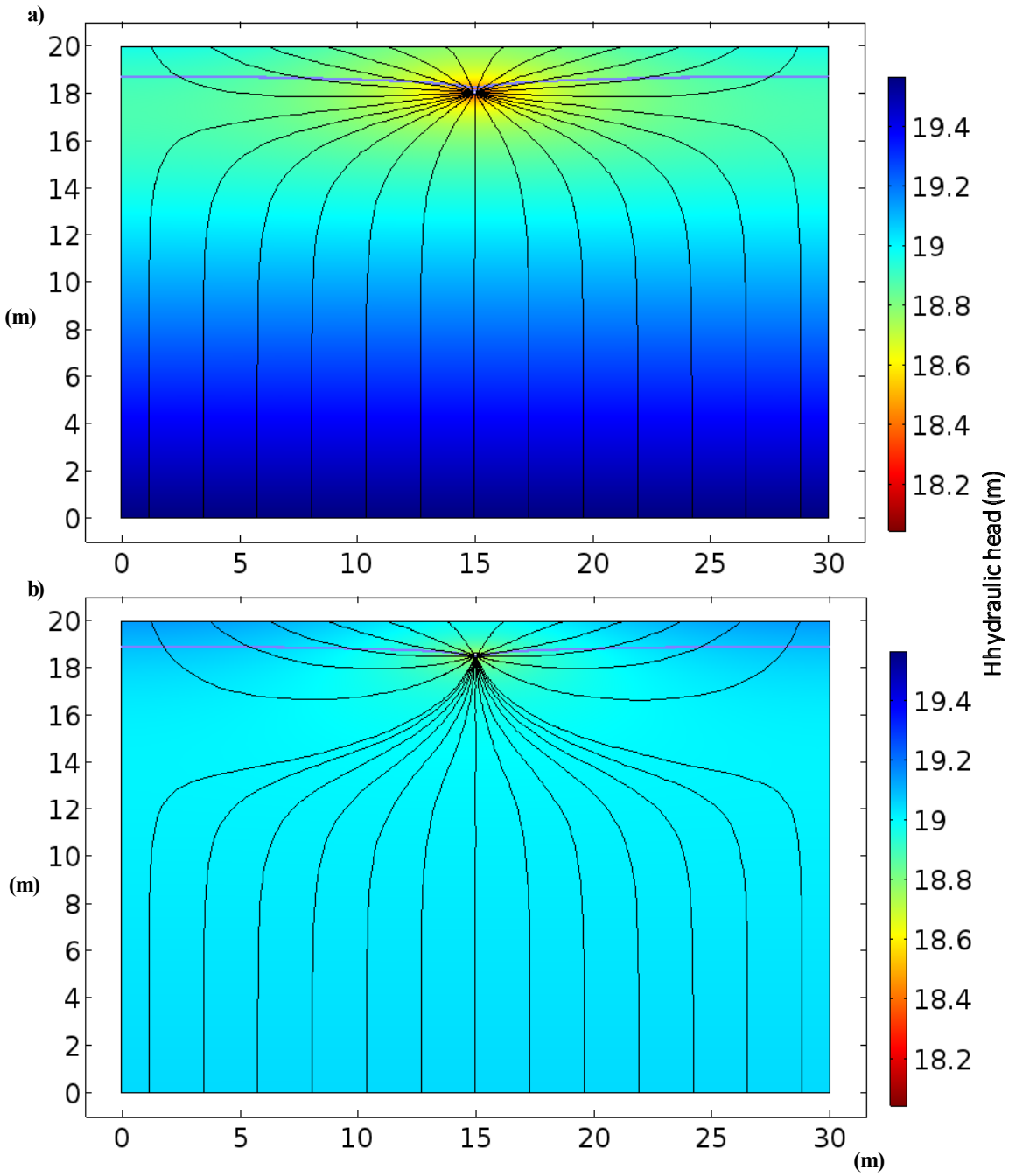


Figure 10: Water table (blue line), hydraulic head (false colors) and flow lines (black lines) around a drain. a) Scenario without SLR, Recharge and Subsidence (related to year 2000). b) Worst scenario with the maximum SLR, Recharge and Subsidence (year 2060).

When an increment of recharge is hypothesized towards increasing sea level scenarios, the discharge through the drains also increases, with an impact on the efficiency of the SD system that must be taken into account. Thus, further calculations of the model have been performed, inclusive of the additional input due to recharge, as described in Section 3.1. The increment of discharge Q_R through the SD system has been calculated in about 0.19, 0.26 and 0.49 L/day·m, corresponding to 1%, 1.5% and 3% of

recharge, which, in turn, increased the piezometric heads due to recharge (ΔG_R) to reach 1.10, 1.50 and 2.80 cm respectively (Table 3).

Finally, the role of subsidence has been quantified. In particular, the reduction of the distance (in terms of depth below the soil level) between the SD and the hydraulic head was considered as a further input to the model. As a result, additional discharge increments of 2.73, 5.74 and 8.69 L/day·m, corresponding to the subsidence values assigned to the years 2020, 2040 and 2060, were obtained.

Considering all the inputs, the numerical findings show a total discharge increment of about 12.50%, 26.97% and 43.35% with respect to the initial Q. Thus, the inclusion of the scenarios studied in this paper in a wider context of water resources management impacts the design of SD systems in a non-negligible way, clearly indicated by the increasing discharge rates to be governed. This result is very relevant for the water management of Nile delta area. In fact, the combination of multiple forcing parameters, such as SLR, artificial subsurface recharge (made to reduce the aquifer salinity) and subsidence require a pre-emptive action in the design and sizing of such fundamental hydraulic structures for the resiliency of agriculture in the area under study.

Table 3: Summary of simulation results for the proposed scenarios. The impact on SD flow increment is highlighted in bold

Time (year)	**2000**	**2020**	**2040**	**2060**
Seal level rise (cm)	0	5.7	14.5	26.7
ΔG_{SLR} (cm)	0	4.65	11.70	21.50
Q_{SLR} (L/day/m)	0	0.81	2.05	3.76
Recharge (%)	0	1	1.5	3
ΔG_R (cm)	0	1.10	1.50	2.80
Q_R (L/day/m)	0	0.19	0.26	0.49
Δ Subsidence	0	16	32	48
Q_{Sub} (L/day/m)	0	2.73	5.74	8.69
Q_{Total} (L/day/m)	29.84	3.73	8.05	12.94
Variation of Q (%)	**0**	**12.50**	**26.97**	**43.35**

4 Conclusions

In this paper, the effect of climate-related drivers on the design of subsurface drainage systems in coastal aquifers is analysed by estimating the flow rate in the draining pipes and its possible increment due to hypothesized conditions of sea level rise, recharge and subsidence. Both surface and subsurface drainage networks in the northern part of the East Nile Delta were designed and realized in the past years, offering an exemplary study area for this research. The evaluations made in this paper are in terms of foreseen scenarios within a time horizon compatible with the life expectancy of the buried draining structure, taking year 2000 as a starting point and years 2020, 2040 and 2060 as reference years for the modelling exercise.

Regarding the climate change effects, the forcing parameters and their assigned values are briefly listed as follows:

i. Based on a "climate-change–driven" acceleration of 0.084 mm/yr^2, sea level rise values of 5.7 cm, 14.5 cm and 26.7 cm (referred to year 2000) were estimated for the years 2020, 2040 and 2060, respectively;

ii. An increment of artificial recharge in order to counteract the incremented sea water intrusion (thus, soil salinity in the root zone) is needed. The hypothesized sea level rise values (5.7 cm, 14.5 cm and 26.7 cm) imply a recharge increment of about 1%, 1.5% and 3%, respectively;

iii. According to recent literature data, subsidence values of 16 cm, 32 cm and 48 cm (referred to year 2000) were calculated for the years 2020, 2040 and 2060, respectively;

Regarding the climate change impacts on the subsurface drainage system (i.e. the increment of flow rate) the outcome of our analyses are:

iv. For what regards the sea level rise, we estimated a discharge increment of about 0.81, 2.05 and 3.76 L/day/m , corresponding to 5.7 cm, 14.5 cm and 23.75 cm of SLR, respectively;

v. For what regards the artificial recharge, a discharge increment of about 0.19, 0.26 and 0.49 L/day/m was evaluated, corresponding to 1%, 1.5% and 3% of recharge, respectively;

vi. For what regards the subsidence, we estimated a discharge increment of about 2.73, 5.74 and 8.69 L/day/m , corresponding to subsidence values of 16, 32 and 48 cm, respectively;

vii. Considering all the forcings (i.e., sea level rise, artificial recharge and subsidence), the numerical findings show a total discharge increment of about 3.73 L/day·m, 8.05 L/day·m and 12.94 L/day/m corresponding to a share of 12.50%, 26.97% and 43.35% with respect to the initial flow rate (year 2000).

This result is very relevant to the future groundwater management in the Nile delta area. In fact, the combination of multiple forcings, such as sea level rise, artificial subsurface recharge (for reducing the aquifer salinity) and subsidence, imply the need for region-specific action plans to minimize the effects of such stressors on the subsurface drainage system. In a wide context of water resources management, the design of subsurface drainage systems must take into account the effects of climate change as a fundamental component of the designing exercise. In particular, this paper demonstrates that the impact of the foreseen conditions on the discharge into the subsurface drainage systems is anything but negligible.

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